

**Korogoro Creek Estuary Management Plan
Survey of Importance of Identified Estuary Values**

PLEASE COMPLETE BOTH SIDES OF THIS SURVEY

PART 1 - VALUES OF KOROGORO CREEK

HOW TO COMPLETE PART 1 OF THE SURVEY:

19 VALUES of the Korogoro Creek estuary system have been identified, many as a result of community feedback during earlier phases of the Estuary Management Planning process. Each VALUE is defined by a short explanation or description to help respondents grade their personal opinion of its importance. These descriptions are important because they may reinforce what you consider important or cause you to rethink your priorities.

After reading through the whole VALUE according to its importance to you. Insert '1' in the box alongside the VALUE if you wish to number it and continue numbering through to a maximum of '19' (or '20' if you suggest a new value in the box provided). However, if you do not wish to number all the boxes simply stop numbering when you are satisfied that all the VALUES of importance to you have been numbered.

ECONOMIC VALUES	IMPORTANCE RANKING
<i>Contribution to local village economy through tourism</i>	<input type="radio"/>
<i>Popular tourism & holiday locality</i>	<input type="radio"/>
<i>Macleay Valley flood security</i>	<input type="radio"/>
SOCIAL VALUES	<input type="radio"/>
<i>High scenic and aesthetic qualities</i>	<input type="radio"/>
<i>Opportunities for recreational line fishing</i>	<input type="radio"/>
<i>Safe swimming location</i>	<input type="radio"/>
<i>Low impact multiple use recreational opportunities</i>	<input type="radio"/>
<i>Cultural heritage and history of the estuary</i>	<input type="radio"/>
<i>Easy vehicle access directly to the estuary banks</i>	<input type="radio"/>
<i>Easy disabled access directly to the estuary</i>	<input type="radio"/>
<i>Easy boating access directly to the estuary</i>	<input type="radio"/>

ENVIRONMENTAL VALUES	IMPORTANCE RANKING
<i>Permanently open creek mouth</i>	<input type="radio"/>
<i>Healthy water quality (within acceptable limits)</i>	<input type="radio"/>
<i>Stable estuary banks</i>	<input type="radio"/>
<i>Healthy riparian (creek bank) vegetation</i>	<input type="radio"/>
<i>Healthy aquatic ecosystems and habitat</i>	<input type="radio"/>
<i>Healthy terrestrial ecosystem and wildlife habitat</i>	<input type="radio"/>
<i>High quality native vegetation communities</i>	<input type="radio"/>
<i>Healthily functioning Swan Pool wetland</i>	<input type="radio"/>
SUGGEST A NEW VALUE <i>Your suggestion...</i>	<input type="radio"/>
Description of value...	

PART 2 - THREATS TO KOROGORO CREEK

HOW TO COMPLETE PART 2 OF THE SURVEY:

10 THREATS to the health and sustainability of the Korogoro Creek estuary system have been identified. Many of these as a result of community feedback during earlier phases of the Estuary Management Planning process. Each THREAT is defined by its potential to negatively impact upon the VALUES listed in PART 1 of this survey (overleaf). The list of VALUES is important because it helps respondents to judge whether or not the threat is significant in terms of the way they value the estuary system.

After reading through the whole of PART 2, please grade each threat according to its importance to you. Insert '1' in the box alongside the THREAT you consider to be of most importance and then continue numbering through to a maximum of '10' (or '19' if you suggest a new threat in the box provided). However, if you do not wish to number all the boxes simply stop numbering when you are satisfied that all the THREATS of importance to you have been numbered.

Korogoro Creek Estuary Management Plan
Survey of Importance of Identified Threats to Estuary Sustainability

THREAT TO SUSTAINABLE ESTUARY MANAGEMENT	VALUES NEGATIVELY IMPACTED	IMPORTANCE RANKING
<i>Lack of funding resources to address identified issues</i>	<ul style="list-style-type: none"> • Healthy water quality • Stable estuary banks • High value native vegetation communities 	<input style="width: 20px; height: 20px;" type="text"/>
<i>Operation of the Macleay Valley flood mitigation scheme</i>	<ul style="list-style-type: none"> • Stable estuary banks • Healthy water quality • Healthy aquatic ecosystems and habitat • Macleay Valley flood security • Healthy functioning Swan Pool wetland • Permanently open creek mouth • Permanently open creek mouth • Macleay Valley flood security 	<input style="width: 20px; height: 20px;" type="text"/>
<i>Potential effect of sea level rise and climate change #</i>	<ul style="list-style-type: none"> • High scenic and aesthetic qualities • High value native vegetation communities • Healthy riparian vegetation • Stable estuary banks • Healthy terrestrial ecosystems and wildlife habitat 	<input style="width: 20px; height: 20px;" type="text"/>
<i>Unmanaged vehicle access to the southern and western creek banks</i>	<ul style="list-style-type: none"> • Healthy water quality • Healthy aquatic ecosystems and habitat 	<input style="width: 20px; height: 20px;" type="text"/>
<i>Inadequate washdown facilities in the vicinity of the boat ramp</i>	<ul style="list-style-type: none"> • Healthy water quality • Healthy aquatic ecosystems and habitat 	<input style="width: 20px; height: 20px;" type="text"/>
<i>Conflict between swimmers and boaters at the estuary mouth</i>	<ul style="list-style-type: none"> • Easy boating access • Safe swimming location • Low impact multiple-use recreational opportunities 	<input style="width: 20px; height: 20px;" type="text"/>
<i>Oyster collection and burleighing downstream of the footbridge</i>	<ul style="list-style-type: none"> • Healthy aquatic ecosystems and habitat • Safe swimming location 	<input style="width: 20px; height: 20px;" type="text"/>
<i>Loss of recreational opportunity due to restrictions on fishing, boating, or vehicle access</i>	<ul style="list-style-type: none"> • Popular tourism/holiday locality • Contribution to local economy through tourism 	<input style="width: 20px; height: 20px;" type="text"/>
<i>Littering in the vicinity of the creek</i>	<ul style="list-style-type: none"> • High scenic and aesthetic qualities • Healthy aquatic ecosystems and habitat 	<input style="width: 20px; height: 20px;" type="text"/>
<i>Lack of community awareness of some issues impacting upon estuary health</i>	<ul style="list-style-type: none"> • Healthy water quality • Healthy aquatic ecosystems and habitat 	<input style="width: 20px; height: 20px;" type="text"/>
<i>Stormwater discharge into the creek</i>	<ul style="list-style-type: none"> • Safe swimming location • Healthy water quality • Healthy aquatic ecosystems and habitat 	<input style="width: 20px; height: 20px;" type="text"/>
<i>Sedimentation / shoaling at the boat ramp</i>	<ul style="list-style-type: none"> • Easy boating access • Safe swimming location 	<input style="width: 20px; height: 20px;" type="text"/>

The full effects of sea level rise and climate change are not currently well understood. However, predicted impacts are likely to be very significant to the functioning of the creek system and barrier dune system which currently protects Hat Head Village.

PLEASE COMPLETE BOTH SIDES OF THIS SURVEY

THREAT TO SUSTAINABLE ESTUARY MANAGEMENT	VALUES NEGATIVELY IMPACTED	IMPORTANCE RANKING
<i>Inadequate water quality testing program</i>	<ul style="list-style-type: none"> • Safe swimming location • Healthy water quality • Healthy aquatic ecosystems and habitat 	<input style="width: 20px; height: 20px;" type="text"/>
<i>Damage to high conservation value native vegetation communities</i>	<ul style="list-style-type: none"> • High scenic and aesthetic qualities • High value native vegetation communities • Healthy terrestrial ecosystems and wildlife habitat 	<input style="width: 20px; height: 20px;" type="text"/>
<i>Inappropriate development</i>	<ul style="list-style-type: none"> • High scenic and aesthetic qualities • Popular tourism/holiday locality • Contribution to local economy through tourism • Cultural heritage and history 	<input style="width: 20px; height: 20px;" type="text"/>
<i>Dumping of garden waste onto the creek bank</i>	<ul style="list-style-type: none"> • High scenic and aesthetic qualities • Healthy riparian vegetation • Healthy terrestrial ecosystems and wildlife habitat 	<input style="width: 20px; height: 20px;" type="text"/>
<i>Weed incursion into riparian vegetation</i>	<ul style="list-style-type: none"> • High value native vegetation communities • Healthy riparian vegetation • Healthy terrestrial ecosystems and wildlife habitat 	<input style="width: 20px; height: 20px;" type="text"/>
<i>Groundwater contamination</i>	<ul style="list-style-type: none"> • Healthy water quality • Healthy aquatic ecosystems and habitat 	<input style="width: 20px; height: 20px;" type="text"/>

SUGGEST A NEW THREAT

Which values are affected by this threat (see overleaf) ...
Your suggestion...

ADD A COMMENT ON THIS SURVEY

Your COMMENTS...

THANK YOU

Please return completed survey to:

Ron Kemsley
Kempsey Shire Council
PO Box 78, West Kempsey NSW 2441

Survey Prepared by Damon Teller, GECO Environmental, T:02 6569 0246 M:0407 878 916 E:damon@westnet.com.au

Stormwater Management Strategy for Hat Head

Prepared as part of the
Korogoro Creek Estuary Management Plan

Prepared for: GECO Environmental
Project Manager: Tim Ruge
Ref: 1106618
Date: October 2008
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PO Box 9
Lennox Head NSW 2478
T 02 6687 7666

PO Box 1446
Coffs Harbour NSW 2450
T 02 6651 7666

info@geolink.net.au

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- A** Stormwater Quality Analysis Results
- B** MUSIC Model Layouts

Background

1.1 Objective

This report has been prepared as part of the Korogoro Creek Estuary Management Plan (EMP). The aim of this report is to outline a stormwater management strategy for Hat Head which:

- identifies the need for stormwater treatment improvements; and
- develops appropriate stormwater treatment concepts and costs to guide future implementation works.

This report has been prepared with consideration of Kempsey Shire Council's (Council) Development Control Plan No. 37 (DCP 37) for Hat Head. One of the stated purposes of DCP 37 is to protect Korogoro Creek from any adverse effects due to stormwater runoff (KSC, 2003:5).

1.2 Urban Characteristics of Hat Head

The village of Hat Head is zoned 2(v) Village under Kempsey Local Environmental Plan 1987. The zoning allows for mainly residential development, with opportunities for minor commercial development to service the local population. The village is predominantly low density single storey residential development. The population of Hat Head is approximately 300 persons. Generally, the urban character consists of little hard paving other than the road surface as indicated in **Plate 1.1** which shows a typical portion of North Hat Head.



Plate 1.1 Aerial Photograph of North Hat Head

1.3 Existing Drainage System

The layout of the existing drainage system is shown in **Illustration 1.1** and **1.2**. The drainage system in North Hat Head comprises grassed swale drainage systems with a network of inlet pits and pipes. The North Hat Head drainage system comprises approximately ten pipe drainage networks which discharge stormwater into Korogoro Creek through separate pipe outlets. A number of smaller sub-catchments exist comprising only swale drains with one pit and a pipe outlet. A total of approximately 17 pipe outlets discharge to Korogoro Creek from the North Hat Head catchment.



Plate 1.2 Typical Street Grassed Swale in North Hat Head



Plate 1.3 Typical Pipe Outlet to Korogoro Creek

The South Hat Head drainage system comprises grassed swale drainage systems in the eastern portion of the residential area and standard kerb and gutter drainage in Marlin Grove and the connecting cul-de-sacs. The South Hat Head catchment discharges to Korogoro Creek at two pipe outlet locations. Drainage from Marlin Grove is directed to an open drain located behind the lots on the western side of the street. This open drain flows through a small pond section, shown in **Plate 1.4** overleaf, before discharging to Korogoro Creek. The eastern portion of the catchment drains to larger swale drainage lines which converge before discharging to Korogoro Creek.

1.4 Rainfall Patterns

The climate at Hat Head is warm and temperate with maritime influences. Mean monthly rainfall data for the area show a strong seasonal pattern with generally wet late summers and early autumns, and dry winters and early springs. The annual rainfall for the area is approximately 1470mm based on records from Smoky Cape near South West Rocks (GECO Environmental, 2007).

1.5 Hydrology

For the purpose of this report Hat Head has been divided into two main catchments: north and south which are divided by Korogoro Creek.

The hydrology of North Hat Head is dominated by the predominantly sandy soils, the existing urban drainage system and the dune system located on the northern side of the town. The sandy soils and the grassed swale drainage system in North Hat Head allow for rapid infiltration of stormwater. Stormwater runoff from the village that does not infiltrate, flows to Korogoro Creek, discharging through a number of pipe outlets.





The catchment area of South Hat Head is partly defined by the high ridgeline to the south. The upper portion of the catchment comprises bushland which is located in Hat Head National Park. Drainage from the catchment discharges to Korogoro Creek at two defined locations.



Plate 1.4 Open Drain and Small Pond



Plate 1.5 Swale Drainage beside Gap Road

1.6 Soils

North Hat Head is located on a beach soil landscape unit defined as the North Shore group (Atkinson, 1999a) which comprise sandy soils with a very high permeability.

The soils of the upper portion of the South Hat Head catchment are an erosional soil landscape unit defined as the Crescent Head group (Atkinson, 1999a) which comprise sandy loam topsoil and sandy clay subsoils.

The lower portion of South Hat Head catchment where the majority of the urban area is located is on an aeolian and barrier soil landscape unit defined as the Korogoro group (Atkinson, 1999a) which comprise sandy soils with a very high permeability.

Existing Stormwater Quality

2.1 Water Quality in Korogoro Creek

The Korogoro Creek Estuary Data Compilation and Processes Study Report (GECO Environmental, 2007) analysed previous data collection conducted for the area from a period of 1994 to 2007. The study concluded that the water quality of the Korogoro Creek is generally within an acceptable range as specified by the relevant ANZECC guidelines. The Creek is a tidal estuary, and so regular tidal flushing reduces the potential for poor water quality from point and diffuse sources within the creek catchment.

Water quality parameters reported in the processes study for Korogoro Creek in the section adjacent to the village include:

- pH levels in the range of 6.0 to 8.0;
- dissolved oxygen levels in the range of 2 to 8 mg/L;
- total suspended solids in the range of 1 to 20 mg/L;
- total phosphorus in the range of 0.01 to 0.3 mg/L;
- total nitrogen in the range of 0 to 4 mg/L; and
- faecal coliforms in the range of 0 to 100 cfu/100mL.

2.2 Stormwater Quality Sampling

To help define existing stormwater quality issues at Hat Head stormwater quality was sampled and analysed. Sampling was undertaken at four stormwater outlets during a rain event on 25 July 2008. Approximately 25 mm of rainfall was recorded at Smoky Cape Lighthouse for the 25 July 2008 with 17 mm on the preceding day. Three of the locations are associated with runoff from Hat Head village and the other sampled runoff from the National Park area to the south as shown in **Illustration 1.1**. The results are summarised in the table below.

Table 2.1 Stormwater Quality Sampling Results for 25 July 2008

<i>Sampling Location¹ / Identification</i>	<i>Faecal Coliforms (cfu/100mL)</i>	<i>Total Phosphorus (mg/L)</i>	<i>Total Nitrogen (mg/L)</i>	<i>Total Oil and Grease (mg/L)</i>
1. Sump at North Hat Head (ID: H08 0372:1664)	2,300	0.10	0.36	3
2. Outlet at North Hat Head (ID: H08 0372:1665)	2,800	0.06	0.19	2
3. Outlet from Nat. Park (ID: H08 0372:1666)	1,500	<0.01	0.80	2
4. Outlet at South Hat Head (ID: H08 0372:1667)	18,000 ²	0.09	0.26	<2

Note: 1. refer to **Illustration 1.1** for location
2. estimated count

The levels for faecal coliforms are in the lower range of values typically expected for residential areas. The value of 18,000 at location No.4 is not considered significantly high in comparison to typical values for residential areas which range from 2,000 to 200,000 cfu/100mL (Fletcher, *et al.* 2004).

The levels for total phosphorus and total nitrogen are generally significantly less than mean stormflow values for residential areas which are expected to be approximately 0.25mg/L and 2.0mg/L respectively (Fletcher, *et al.* 2004). The values in **Table 2.1** are more representative of typical values from bushland areas. These low pollutant values are thought to be due to the low density of development and the treatment provided by the grassed swales. This is discussed in more detail in **Section 3**.

Similar to above, the levels for total oil and grease are also generally significantly less than typical values for residential areas which range from 3mg/L to 20mg/L (Fletcher, *et al.* 2004).

Samples were also analysed for metals. No significant readings were recorded.

The analysis results for the sampling are contained in **Appendix A**.

2.3 Stormwater Issues

2.3.1 Community Concerns

Community members raised a number of issues at a consultation meeting in August 2006. The issues include the lack of gross pollutant traps on stormwater pipes discharging directly to the creek and pollution caused by the washing of vehicles and caravans in the caravan park (GECO Environmental, 2007).

2.3.2 Site Observations

Potential sources of stormwater pollutants observed during a site inspection on 9 September 2008 include leaf litter and lawn clippings as indicated in the photographs below, and some minor incidences of litter such as plastic bags and drink cans. However the latter may be the result of littering directly along the creek bank.



Plate 2.1 Leaf Litter in Caravan Park Drains



Plate 2.2 Lawn Clippings

There were no significant erosion issues observed at the various stormwater outlets.

2.3.3 Interpretation of Sampling Results

The stormwater sampling described in **Section 2.2** indicates no major issues in regard to the quality of the stormwater from Hat Head village. The results are generally in the lower range of values typically expected for residential areas.

2.3.4 Future Development

South Hat Head currently contains undeveloped residential lots. There are also areas of adjoining land zoned 'Rural 1D Investigation' which may be rezoned and developed into residential allotments in the future. Development of these allotments will increase the stormwater pollutant load and runoff rate from the South Hat Head catchment.

The amount of impervious area within residential allotment in both North and South Hat Head may also increase with redevelopment / house renovations. This will increase the stormwater pollutant load and runoff rate from the allotments.

2.3.5 Summary of Stormwater Issues

The above indicates that current stormwater quality issues for Hat Head are relatively minor in comparison to typical residential areas. The types of pollutants are typical of residential areas: litter, sediments, oils from cars, and nutrients from different sources such as grass clippings, attached to sediment and potentially from fertilisers used on lawns.

The low pollutant values are thought to be due to the low density of development and treatment and infiltration provided by the grassed buffer strips and swales.

The main issues that may impact on stormwater quality are:

- future development in South Hat Head without appropriate detention and treatment measures;
- increased roof areas and paved areas from redevelopment throughout Hat Head without appropriate detention and treatment measures; and
- replacement of the existing grassed swales and buffer strips with standard kerb and gutter street drainage.

The following section outlines and assesses a number of stormwater management strategies that address the above issues.

Stormwater Management Strategies

The following stormwater issues and strategies are investigated in this section to determine the impact on Korogoro Creek:

1. future development in South Hat Head (refer to **Section 3.2**);
2. redevelopment throughout Hat Head (refer to **Section 3.3**); and
3. retain grassed swales as opposed to replacing the swales with standard kerb and gutter street drainage (refer to **Section 3.4**);

Gross pollutant traps and educational strategies are also discussed in **Section 3.5** and **Section 3.6** respectively. Educational strategies are discussed in regard to addressing issues such as litter, grass clippings and car washing detergents.

3.1 MUSIC Modelling

Assessment of the issues and strategies has included water quality modelling using the MUSIC program (Model for Urban Stormwater Improvement Conceptualisation - Version 3.01). The objective of the water quality modelling is to establish a model of existing conditions to help assess the impact of future works on stormwater quality.

The MUSIC model requires recorded rainfall data at six minute time steps to more accurately model the impact of small treatment units such as grassed swales. Coffs Harbour pluviograph rainfall data (6-minute interval data) has been used in the model as this was the nearest pluviograph data that provided a reasonable continuous record over a period of ten years.

The land use types used in the MUSIC model were separated into the following categories:

- urban residential – roads (30% impervious): comprising the bitumen area and the adjoining grassed road reserve area;
- urban residential – roofs (100% impervious): modelling of the roofs separate from the remaining portions of the lots enabled assessment of the impacts of installing rainwater tanks;
- urban residential – other areas (5% impervious): the area of each lot excluding the dwellings. An allowance of 5% impervious area accounts for paved areas within each lot;
- rural residential (20% impervious): the larger lots in South Hat Head; and
- forest: the vegetated dune area in North Hat Head and the bushland on the upper slopes of South Hat Head.

The impervious percentages listed above are based on estimates obtained from aerial photography of Hat Head. The rainfall-runoff parameters and stormwater quality parameters used in the MUSIC model are based on parameter values recommended in *MUSIC Modelling Guidelines* produced by Gold Coast City Council (2006).

3.2 Future Development in South Hat Head

3.2.1 Existing Conditions

To establish a MUSIC model of existing conditions South Hat Head was separated into two main sub-catchments, S1 and S2 shown in **Illustration 3.1**, which approximate the main drainage networks. The areas within each sub-catchment were grouped into the respective land use types listed in **Section 3.1**. The swales were modelled with a seepage loss of 180 mm/hour. This figure is based on the lower end of MUSIC default seepage values for sandy soils. The layouts of the MUSIC models are shown in **Appendix B**.

The mean value of pollutant concentrations flowing to Korogoro Creek from sub-catchment S2 are shown in **Table 3.1** together with the sampling results for the outlet from the sub-catchment (Outlet No.4). The mean values and standard deviations obtained from the MUSIC model are generally in agreement with the sampling results. The mean values plus the standard deviation appear to generally reflect the sampled values. Therefore it is considered that the MUSIC model provides an acceptable representation of the existing conditions at South Hat Head.

Table 3.1 Stormwater Pollutant Concentrations for South Hat Head – Comparison of Modelled and Sampled Values

	<i>Total Suspended Solids (mg/L)</i>		<i>Total Phosphorus (mg/L)</i>		<i>Total Nitrogen (mg/L)</i>	
	<i>Modelled</i>	<i>Sampled</i>	<i>Modelled</i>	<i>Sampled</i>	<i>Modelled</i>	<i>Sampled</i>
Maximum Value	180	-	0.37	0.09	1.86	0.26
Mean Value and Std Dev.	0.48± 4.55	-	0.01± 0.02	0.09	0.03 ± 0.21	0.26

3.2.2 Future Development

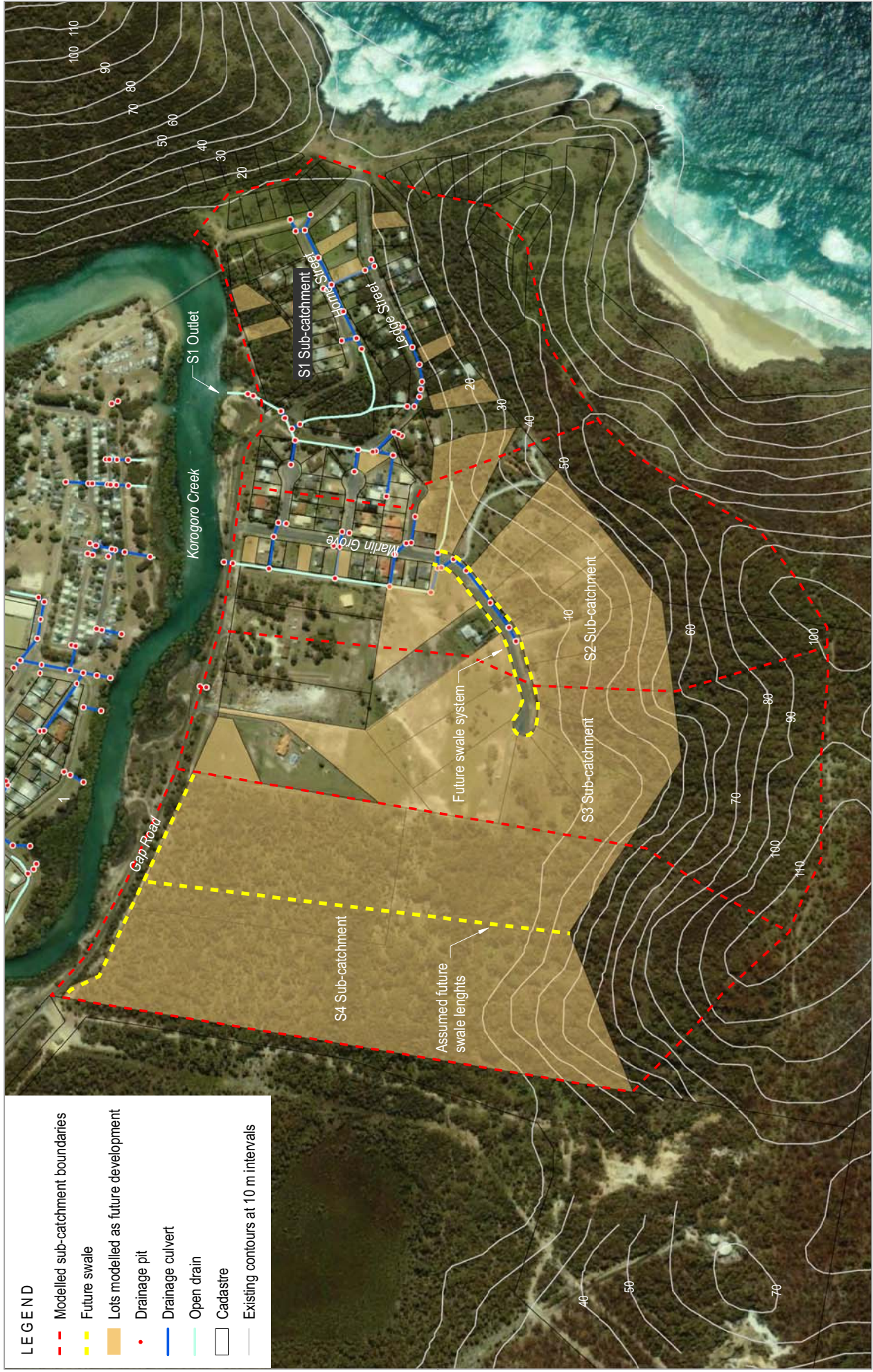
Future development has been assessed separately for each sub-catchment shown in **Illustration 3.1**. The impact of future / infill development has been assessed by comparison of the mean annual pollutant load for existing conditions and future development conditions. This is discussed for each sub-catchment below.

Sub-Catchment S1

Future development for this sub-catchment comprises infill development of eight standard residential lots. These lots were modelled as either a 'forest' or 'rural residential' node for existing conditions. The future development scenario modelled the additional lots by increasing the area of the 'urban residential – roofs' node by 250 m² per additional lot and increasing the area of the 'urban residential – other areas' node by the remaining area of these lots. The impact of the infill development on pollutant increases is relatively small due to the effectiveness of the existing swale system. The effectiveness of the swale system is still considered very high in treating the future development of the sub-catchment. Therefore no additional works are considered necessary to treat the small amount of infill development.

Table 3.2 Sub-Catchment S1 – Changes in Annual Pollutant Loads for Infill Development

<i>Parameter</i>	<i>Units</i>	<i>Existing Conditions</i>	<i>Future – Infill Development</i>	<i>Percentage Increase in Load</i>
Flow	ML/year	31.2	31.9	2%
Total Suspended Solids	kg/year	2,780	2,860	3%
Total Phosphorus	kg/year	6.4	6.7	5%
Total Nitrogen	kg/year	45.5	47.3	4%



Sub-Catchment S2

Future development for this sub-catchment comprises infill development of four standard residential lots and nine rural residential lots. The future standard residential lots were modelled as 'rural residential' for existing conditions. The model for future development accounts for these lots with an increased area of the 'urban residential – roofs' node and the 'urban residential – other areas' node as described previously.

The additional rural residential lots were modelled as 'forest' for existing conditions and converted to 'rural residential' nodes for the future scenario.

The increase in pollutant load from the infill development is relatively large in regard to the MUSIC results. However, this is considered an overestimate due to the way in which it was modelled. Changing the status of the additional rural residential lots from 'forest' to 'rural residential' nodes will overestimate the increase in pollutant load. The overall effectiveness of the existing swale system in treating the future pollutant loads remains in the order of 70% removal efficiency which is still considered relatively high removal efficiency. To increase this removal rate it is recommended that a swale drainage system is constructed along the length of Marlin Grove where the future rural residential development will occur.

Table 3.3 Sub-Catchment S2 – Changes in Annual Pollutant Loads for Infill Development

<i>Parameter</i>	<i>Units</i>	<i>Existing Conditions</i>	<i>Future – Infill Development</i>	<i>Percentage Increase in Load</i>
Flow	ML/year	38.6	40.1	4%
Total Suspended Solids	kg/year	3,010	3,980	32%
Total Phosphorus	kg/year	5.88	9.29	58%
Total Nitrogen	kg/year	49.1	76.5	56%

Sub-Catchment S3

Sub-catchment S3 has been modelled as comprising 'forest' and 'rural residential' nodes. Future development comprises reducing the 'forest' node and the 'rural residential' node by 4.4ha to account for approximately four new rural residential lots.

The increase in pollutant load is similar to Sub-catchment S2. Similarly the increase is considered an overestimate and the overall effectiveness of the existing swale system in treating the future pollutant loads remains in the order of 65% removal efficiency which is still considered relatively high removal efficiency. The previously recommended swale drainage system in Marlin Grove will assist in reducing the impact of the development.

Table 3.4 Sub-Catchment S3 – Changes in Annual Pollutant Loads for Rural Residential Development

<i>Parameter</i>	<i>Units</i>	<i>Existing Conditions</i>	<i>Future – Infill Development</i>	<i>Percentage Increase in Load</i>
Flow	ML/year	39.3	39.5	1%
Total Suspended Solids	kg/year	3,250	3,740	15%
Total Phosphorus	kg/year	6.64	8.34	26%
Total Nitrogen	kg/year	59.3	72.4	22%

Sub-Catchment S4

This sub-catchment is currently zoned 1(d) 'rural investigation' in the areas highlighted in **Illustration 3.1**. Potential development of this area has been modelled by converting 19ha (the shaded area in **Illustration 3.1**) from a 'forest' node to a 'rural residential' node. The existing scenario was modelled without any treatment system such as swales. The developed scenario was modelled with and without treatment to assess the impact of a swale system. The treated development scenario shown in **Table 3.5** assumes a total length of 800 m of swales for the development area which approximates a length of swale along the frontage and depth of this area as shown in **Illustration 3.1**. This length of swale is considered adequate to treat the developed area.

Table 3.5 Sub-Catchment S4 – Changes in Annual Pollutant Loads for Rural Residential Development

<i>Parameter</i>	<i>Units</i>	<i>Existing Conditions</i>	<i>Future Development – No Treatment</i>	<i>Future Development – With Treatment</i>	<i>Percentage Increase in Load</i>
Flow	ML/year	142	155	31.5	-78%
Total Suspended Solids	kg/year	6,230	14,500	3,310	-47%
Total Phosphorus	kg/year	7.09	37.4	7.99	13%
Total Nitrogen	kg/year	80.7	331	67.4	-16%

3.2.3 Costs

Construction cost estimates associated with the above works are:

- Sub-Catchment S1 – no additional works therefore no costs;
- *Sub-Catchment S2 and S3* - recommended swale drainage system on both sides of Marlin Grove where the future rural residential development will occur. Construction cost estimate for total length of swale of 600m with top width of 5m is approximately \$200,000;
- *Sub-Catchment S4* - recommended swale drainage system with a total length of swale of 800m for the development area. Construction cost estimate assuming top width of 5m is approximately \$250,000.

3.3 Redevelopment throughout Hat Head

Redevelopment of existing residential lots associated with house renovations / demolition and rebuilding will generally result in increased roof areas and paved areas. The impact on stormwater quality has been assessed with consideration of the following scenarios:

- redevelopment without any additional detention and treatment measures; and
- redevelopment with rainwater tanks and reuse requirements for each redeveloped allotment.

This issue has been assessed by modelling a typical drainage catchment in North Hat Head. North Hat Head has been selected because it is considered that more redevelopment is likely to occur in this area.

3.3.1 Existing Conditions

North Hat Head was separated into ten sub-catchments which approximate the main drainage networks in the village. One typical sub-catchment has been modelled to assess this issue. The areas within each sub-catchment were grouped into one of the respective land use types listed in **Section 3.1**. An example of the model layout for three sub-catchments is shown in **Appendix B**.

Buffer areas were modelled for the runoff from the roads, roofs and other portions of the lots to simulate overland flow across the grassed areas before the flows enter the swales. A seepage loss of 180 mm/hour was modelled over the buffer areas and in the swales based on the lower end of default values for sandy soils.

The mean value of pollutant concentrations flowing to Korogoro Creek are shown in **Table 3.2** together with the sampling results from **Section 2.2**. The mean values obtained from the MUSIC model are generally in agreement with the sampling results. The mean values less the standard deviation appear to generally reflect the sampled values. Therefore it is considered that the MUSIC model provides an acceptable representation of the existing conditions at Hat Head.

Table 3.6 Stormwater Pollutant Concentrations for North Hat Head – Comparison of Modelled and Sampled Values

	<i>Total Suspended Solids (mg/L)</i>		<i>Total Phosphorus (mg/L)</i>		<i>Total Nitrogen (mg/L)</i>	
	<i>Modelled</i>	<i>Sampled</i>	<i>Modelled</i>	<i>Sampled</i>	<i>Modelled</i>	<i>Sampled</i>
Maximum Value	10 - 100	-	0.13 – 0.16	0.06 – 0.10	1.4 – 1.6	0.19 – 0.36
Mean Value and Std Dev.	15 ± 1.1	-	0.13 ± 1.0	0.06 – 0.10	1.4 ± 1.1	0.19 – 0.36

3.3.2 Future Redevelopment

Future redevelopment has been assessed by comparison of the mean annual pollutant load for existing and future redevelopment conditions. The following assumptions have been used in the MUSIC modelling:

- redevelopment will involve an increase in roof area from approximately 300m² to 350m² per lot;
- redevelopment will involve an increase in impervious paved area from 5% to 20% for the remaining area of each lot (the total lot area less the roof area);
- modelling of rainwater tanks has assumes installation of a 4,000 L rainwater tank for each redeveloped lot with 50% of the capacity dedicated to stormwater detention and remainder used for reuse purposes including toilet flushing.

The above stormwater detention allowance provided by the rainwater tank is considered sufficient in respect to the DCP 37 requirement of no increase in pre-development flow rates during a 1 in 5 year storm event (Section 4.13c in DCP 37).

Initially the impact of redevelopment was assessed without the inclusion of rainwater tanks. The impact of the increased roof and paved areas increases the pollutant load reaching Korogoro Creek by approximately 10 to 15% as shown overleaf in **Table 3.7**. This increase at the outlet to Korogoro Creek is not very large due to the effectiveness of the existing swale system.

Modelling of redevelopment with the inclusion of rainwater tanks is shown overleaf in **Table 3.8**. The impact of rainwater tanks is not very significant when comparing the quality at the outlet to Korogoro Creek. Comparing the results in **Table 3.7** and **Table 3.8** indicates that the annual pollutant load to Korogoro Creek from the redeveloped scenario is only reduced by approximately 3% with the requirement of rainwater tanks. This seemingly small impact is due to the masking effect of the high treatment efficiency of the existing swale system which can adequately treat the additional loadings from redevelopment without the need for rainwater tanks. However, the direct impact of rainwater tanks in reducing annual pollutant loads is in the range of 5% to 15% prior to treatment by the swale system.

Table 3.7 **Redevelopment – Changes in Annual Pollutant Loads Without Additional Treatment Measures**

<i>Parameter</i>	<i>Units</i>	<i>Existing Conditions</i>	<i>Future Redevelopment (No Rainwater Tanks)</i>	<i>Percentage Increase in Load</i>
Flow	ML/year	0.341	0.384	13%
Total Suspended Solids	kg/year	5.03	5.65	12%
Total Phosphorus	kg/year	0.045	0.051	12%
Total Nitrogen	kg/year	0.478	0.541	13%

Table 3.8 **Redevelopment – Changes in Annual Pollutant Loads With Rainwater Tanks**

<i>Parameter</i>	<i>Units</i>	<i>Existing Conditions</i>	<i>Future Redevelopment (With Rainwater Tanks)</i>	<i>Percentage Increase in Load</i>
Flow	ML/year	0.341	0.374	10%
Total Suspended Solids	kg/year	5.03	5.49	9%
Total Phosphorus	kg/year	0.045	0.049	9%
Total Nitrogen	kg/year	0.478	0.525	10%

Regardless of the relatively small impact of rainwater tanks in comparison to the existing swale system the requirement of rainwater tanks or some other form of on-site detention and treatment is recommended. This will still have some impact in reducing the overall load to Korogoro Creek and also significantly reduce the reliance on the existing swale system to provide all necessary treatment.

3.3.3 Costs

Construction cost estimates associated with the rainwater tanks are approximately \$2,000 per lot.

3.4 Retaining Grassed Swales

Council's Development Control Plan for Hat Head (DCP 37) states that Council does not intend providing or requiring the construction of any kerb and guttering (KSC, 2003:13) and lists the lack of kerb and guttering as an important landscape feature that requires protection by "maintaining a stormwater system of grassed swales" (KSC, 2003:11).

The previous assessment in **Section 3.3** shows the high treatment effectiveness of the existing swale system in buffering any increases in stormwater pollutant generation associated with redevelopment. The purpose of the following modelling is to provide a more focussed assessment of the benefit of the existing swale system in comparison to a kerb and gutter system in regard to stormwater pollution.

The impact of kerbs and guttering is very significant when comparing to existing conditions. The results in **Table 3.9** indicate that the annual pollutant load to Korogoro Creek from the redeveloped scenario is increased in the order of 100 times. In order to mitigate this effect, a significant area would be required for treatment on each residential lot and at the outlets. This is not considered feasible due to the location of

the outlets in mangroves. The significant impact of kerbs and guttering highlights the high level of effectiveness of the existing swale system.

Table 3.9 **Redevelopment – Changes in Annual Pollutant Loads With Kerbs and Guttering**

<i>Parameter</i>	<i>Units</i>	<i>Existing Conditions</i>	<i>Future Redevelopment (With Kerbs and Guttering)</i>
Flow	ML/year	0.522	52.2
Total Suspended Solids	kg/year	19.3	8,400
Total Phosphorus	kg/year	0.0804	16.6
Total Nitrogen	kg/year	0.685	77.7

3.5 Gross Pollutant Traps

Community concerns in regard to stormwater pollution include the lack of gross pollutant traps (GPT's) on stormwater pipes discharging directly to the creek. Gross pollutants are large pieces of debris including litter. The use of gross pollutants can be an expensive option.

The most appropriate type of GPT for Hat Head would be in the form of an end-of-pipe litter net as that indicated in **Plate 3.1**. These types of nets have a reported 50-75% pollution reduction efficiency.

Maintenance of the nets involves Council manually removing the nets from the pipes, emptying the nets into a truck and reattaching the nets to the pipe. This could be undertaken by manually lifting the nets due to the relatively small size required for each pipe outlet at North Hat Head.



Source: http://www.stormwater.com.au/end_pipe.htm
Plate 3.1 **End of Pipe Litter Net**

The effectiveness of using a litter net at Hat Head is not considered high due to the following reasons:

- the relatively low litter generation from Hat Head;
- a significant portion of the litter present in the creek may be due to individuals dropping the litter near the creek edge during recreational activities such as walking, canoeing, swimming and fishing. This litter will not be captured by the nets; and
- litter originating in the drainage system will often be 'caught' in the grassed swale systems or grated drainage inlet pits.

3.5.1 Costs

Cost estimates for supply and installation of litter nets are approximately \$2,500 per net with one net required per outlet pipe. If this option was pursued it is assumed that up to eight nets may be required at a cost of approximately \$20,000. This does not include maintenance costs.

3.6 Educational Strategies

Education can form a key source control tool for dealing community concerns regarding washing of vehicles and caravans in the caravan park and gross pollutants in the form of litter. Education can be an appropriate and effective strategy for minimising these forms of stormwater pollution because it:

- targets the individual: the behaviour of individuals associated with littering, washing vehicles and caravans, and collecting lawn clippings;
- targets diffuse sources: the 'diffuse source' nature of stormwater pollution such as littering means that structural solutions are often less effective than education;
- impacts on the whole community: education has the capacity to mobilise the whole community which can assist in further targeting of individuals by the community;
- is a major motivator: simple messages such as 'stormwater flows untreated into Korogoro Creek' can be a motivator for appropriate behaviour.

Educational strategies targeting the issue of littering at Hat head may include a stage approach involving:

- initially targeting education of people about the impacts of littering on the local environment and promoting the concept of individuals taking responsibility for their own litter (minimising the amount of litter they produce and taking what they produce home). This phase may involve signage works; and
- depending on the success of the initial education, installation of strategically located litter bins maintained by Council.

3.6.1 Costs

The cost of an educational strategy similar to that described above is estimated at approximately \$50,000 which includes fees for development of a strategy, community and stakeholder consultation, and development, manufacture and installation of signage and litter bins.