

CONSTRUCTION SPECIFICATION

C213

EARTHWORKS

SPECIFICATION C213 - EARTHWORKS

CLAUSE	CONTENTS	PAGE
CITATION		4
ORIGIN OF DOCUMENT, COPYRIGHT		4
VERSIONS, C213 - Earthworks		4
GENERAL.....		5
C213.01	SCOPE	5
C213.02	REFERENCE DOCUMENTS	5
C213.03	ABBREVIATIONS	6
C213.04	DEFINITIONS	6
C213.05	EARTHWORKS MATERIALS	6
C213.06	PROTECTION OF EARTHWORKS	7
C213.07	SETTING OUT OF EARTHWORKS	7
C213.08	STOCKPILE SITES	8
REMOVAL OF TOPSOIL		8
C213.09	SCOPE	8
C213.10	TOPSOIL STOCKPILES	9
CUTTINGS.....		9
C213.11	SCOPE	9
C213.12	EXCAVATION	9
C213.13	BATTER TOLERANCES	10
C213.14	BENCHING IN CUTTINGS.....	10
C213.15	TREATMENT OF FLOORS OF CUTTINGS.....	11
C213.16	TRANSITION FROM CUT TO FILL.....	11
BLASTING.....		13
C213.17 GENERAL		13
C213.18	PRESPLITTING	14
C213.19	BLASTING RECORDS	14
C213.20	CONTROL OF AIR BLAST OVER-PRESSURE	15
C213.21	CONTROL OF GROUND VIBRATION	15
UNSUITABLE MATERIAL.....		16
C213.22	GENERAL	16
EMBANKMENT CONSTRUCTION		17

C213.23	SCOPE	17
C213.24	EMBANKMENT MATERIAL.....	17
C213.25	FOUNDATIONS FOR EMBANKMENTS	17
C213.26	HILLSIDE EMBANKMENTS.....	18
C213.27	PLACING FILL FOR EMBANKMENT CONSTRUCTION.....	19
C213.28	EMBANKMENT BATTERS	20
C213.29	ROCK FACING OF EMBANKMENTS.....	20
C213.30	TRIMMING TOPS OF EMBANKMENTS	20
C213.31	SELECTED MATERIAL ZONE.....	21
C213.32	FILL ADJACENT TO STRUCTURES	21
C213.33	TREATMENT AT WEEPHOLES	22
C213.34	SELECTED BACKFILL	22
C213.35	SPOIL.....	23
C213.36	BORROW.....	23
COMPACTION AND QUALITY CONTROL.....		24
C213.37	COMPACTION AND MOISTURE REQUIREMENTS	24
C213.38	TEST LOCATIONS	26
C213.39	DEFLECTION MONITORING.....	26
C213.40	WIDENING OF FORMATION	26
LIMITS AND TOLERANCES.....		27
C213.41	SUMMARY OF TOLERANCES.....	27
ANNEXURE C213A.....		28

CITATION

This document is named “Kempsey Shire Council, Construction Specification C213 – Earthworks”.

ORIGIN OF DOCUMENT, COPYRIGHT

This document was originally based on PMHC AUS-SPEC. Parts of the AUS-SPEC document that remain are still subject to the original copyright.

VERSIONS, C213 - Earthworks

VERSION	AMENDMENT DETAILS	CLAUSES AMENDED	DATE ISSUED (The new version takes effect from this date)	Authorised by the Director of Infrastructure
1.0	Version 1 – First Draft Version		March 2025	

SPECIFICATION C213 : EARTHWORKS

GENERAL

C213.01 SCOPE

- | | | |
|----|---|----------------|
| 1. | The work to be executed under this Specification consists of:- | <i>Scope</i> |
| | <ul style="list-style-type: none"> (a) removal of topsoil (b) all activities and quality requirements associated with site regrading, the excavation of cuttings, the haulage of material and the construction of embankments to the extent defined in the Approved Plans and Specification. (c) removal and replacement of any unsuitable material, (d) any spoil or borrow activities associated with earthworks, and (e) any additional processing of selected material for the selected material zone. | |
| 2. | Requirements for quality control and testing, including maximum lot sizes and minimum test frequencies, are cited in the Development Construction Specification – Quality Control Requirements (CQC). | <i>Quality</i> |

C213.02 REFERENCE DOCUMENTS

- | | | |
|-----|---|---|
| 1. | Documents referenced in this specification are listed in full below whilst being cited in the text in the abbreviated form or code indicated. | <i>Documents
Standards Test
Methods</i> |
| (a) | Council Specifications <ul style="list-style-type: none"> D2 - Pavement Design C201 - Control of Traffic C211 - Control of Erosion and Sedimentation C212 - Clearing and Grubbing C220 - Stormwater Drainage – General C241 - Stabilisation C273 - Landscaping | |
| (b) | Australian Standards <ul style="list-style-type: none"> AS 1289.3.3.1 - Methods of testing soils for engineering purposes - Soil classification tests - Calculation of the plasticity index of a soil. AS 1289.5.1.1 - Methods of testing soils for engineering purposes - Soil compaction and density tests – Determination of the dry density/moisture content relation of a soil using standard compactive effort. AS 1289.5.4.1 - Methods of testing soils for engineering purposes - Soil compaction and density tests – Compaction Control Test - Dry density ratio, moisture variation and moisture ratio. | |

- AS 1289.5.7.1 - Methods of testing soils for engineering purposes - Soil compaction and density tests - Compaction control test - Hilf density ratio and Hilf moisture variation (rapid method).
 - AS 1289.6.1.1 - Methods of testing soils for engineering purposes - Soil strength and consolidation tests – Determination of the California Bearing Ratio of a soil – Standard laboratory method for a remoulded specimen.
 - AS 2187 - Explosives - Storage, transport and use (SAA Explosive Code)
 - Part 1 Storage and land transport
 - Part 2 Use of explosives
 - AS 3798 - Guidelines on earthworks for commercial and residential developments
- (c) Other
- Safe Work Australia - Australian Code for the Transport of Explosives by Road and Rail
- (d) Other
- (e) Standard Drawings

C213.03 ABBREVIATIONS

C213.04 DEFINITIONS

Unsuitable Material Unsuitable material is any material as defined in Section 4.3 of AS 3798-2007 or as identified by the Principal Certifier in conjunction with a qualified Geotechnical Engineer. Unsuitable material must be removed and replaced with a suitable material to provide a reasonable platform for subsequent earthworks.

C213.05 EARTHWORKS MATERIALS

(b) Earthworks Materials

1. The Principal Contractor shall be responsible for any assumptions made in relation to the nature and types of the materials encountered in excavations and the bulking and compaction characteristics of materials incorporated in embankments.
2. Where material from excavations is acceptable for use in embankments, but the Principal Contractor elects to:-

***Material
Characteristics***

***Embankment
Material
Deficiency***

- (a) Spoil it, or
- (b) Use it for the Principal Contractor's own purposes, or
- (c) Use it as a source of pavement materials, or
- (d) Construct embankments with dimensions in excess of those specified.

and a deficiency of material for embankment construction is thereby created, the Principal Contractor shall make good that deficiency from sources of material meeting the quality requirements specified in Clause C213.23. The cost of making good such deficiency of material shall be borne by the Principal Contractor.

Quality

C213.06 PROTECTION OF EARTHWORKS

1. The Principal Contractor's responsibility for care of the Works shall include the protection of earthworks.

Principal Contractor's Responsibility

2. The Principal Contractor shall install effective erosion and sedimentation control measures in accordance with Specification C211 CONTROL OF EROSION AND SEDIMENTATION, prior to commencing earthworks, and shall maintain these control measures for the duration of the works.

Erosion and Sedimentation Control

3. Adequate drainage of all working areas shall be maintained throughout the period of construction to ensure run-off of water without ponding, except where ponding forms part of a planned erosion and sedimentation control system.

Drainage of Working Areas

4. When rain is likely or when work is not proposed to continue in a working area on the following day, precautions shall be taken to minimise ingress of any excess water into earthworks material. Ripped material remaining in cuttings and material placed on embankments shall be sealed off by adequate compaction to provide a smooth tight surface.

Wet Weather Precautions

5. Should insitu or stockpiled material become over wet as a result of the Principal Contractor not providing adequate protection of earthworks, the Principal Contractor shall be responsible for replacing and/or drying out the material and for any consequent delays to the operations.

Wet Material

C213.07 SETTING OUT OF EARTHWORKS

1. Before earthworks operations commence and after survey controls are in place, batter profiles shall be established by the Principal Contractor and the necessary pegs driven at 25 m intervals or at each cross section shown on the Approved Plans, whichever is the lesser or at other interval(s) as required by the Principal Certifier. The chainage/station, offset from control line and slope distance to finished surface level, shall be clearly marked on each peg.

Batter Profiles

2. The batter profiles shall be repositioned by the Principal Contractor at each change in the slope of the batter and at intervals of not more than 5 m of vertical height.

Profile Location

3. All pegs and batter profiles shall be maintained in their correct positions. They shall be removed by the Principal Contractor on completion of subdivision works.

Retention and Removal of Pegs

- | | | |
|----|---|---|
| 4. | The foregoing shall be the minimum requirement. Additional pegs and profiles may be required to suit the Principal Contractor. These shall not be painted with the same colours used for the specified setting out pegs and stakes. | <i>Additional Pegs</i> |
| 5. | The position and extent of all transitions from cuttings to embankments and foundations for shallow embankments shall be marked with clearly labelled stakes in accordance with Clauses C213.15 and C213.24. | <i>Transitions
Cuttings/
Embankments</i> |

C213.08 STOCKPILE SITES

- | | | |
|----|---|--|
| 1. | The Principal Contractor shall obtain the written consent of the Principal Certifier and all associated Environmental Approvals to use any stockpile site which is not shown on the Approved Plans. Proposals in this regard shall be submitted at least three working days (3) before stockpiling is due to commence and shall specify the maximum dimensions of the proposed stockpile. | <i>Additional
Stockpile Sites</i> |
| 2. | Any clearing and grubbing required for these sites shall be carried out in accordance with Specification C212 CLEARING AND GRUBBING. Temporary erosion and sedimentation control measures shall be taken in accordance with Specification C211 CONTROL OF EROSION AND SEDIMENTATION. | <i>Clearing and
Grubbing</i> |
| 3. | Restoration of stockpile sites following completion of the work shall be carried out in accordance with Specification C273 LANDSCAPING. | <i>Restoration</i> |
| 4. | Stockpiles shall not be located within the canopy line of trees to be retained as shown on the approved plans. | |

REMOVAL OF TOPSOIL

C213.09 SCOPE

- | | | |
|----|--|------------------------------|
| 1. | Topsoil is surface soil which is reasonably free from subsoil, refuse, clay lumps and stones. | <i>Definition</i> |
| 2. | Removal of topsoil on any section of the Works shall only commence after erosion and sedimentation controls have been implemented in accordance with Specification C211 CONTROL OF EROSION AND SEDIMENTATION and when clearing, grubbing and disposal of materials have been completed in accordance with Specification C212 CLEARING AND GRUBBING on that section of the Works. | <i>Prerequisites</i> |
| 3. | Topsoil throughout the length of the work shall be removed and stockpiled separately clear of the work with care taken to avoid contamination by other materials. | <i>Extent of Work</i> |

The work shall include the following:-

- (a) **Cuttings**
Removal of the topsoil to a depth quoted in Annexure C213A or as directed by the Principal Certifier.
- (b) **Embankments**

Removal of topsoil over the base of embankments up to the depth below the natural surface quoted in Annexure C213A, or as directed by the Certifying Engineer. For those embankments or sections of embankment where the height of embankment from natural surface to underside of pavement is less than two metres, topsoil which is deeper than the depth quoted in Annexure C213A shall be removed to its full depth as directed by the Principal Certifier.

(c) **Other Locations**

Removal of topsoil as directed by the Principal Certifier.

C213.10 TOPSOIL STOCKPILES

- | | |
|---|---------------------------|
| 1. The maximum height of stockpiles shall not exceed 2.5 m and the maximum batter slope shall not exceed 2:1. | <i>Height and Batter</i> |
| 2. Topsoil stockpiles shall not contain any timber or other rubbish and shall be trimmed to a regular shape. | <i>Stockpiles Trimmed</i> |
| 3. To minimise erosion, stockpile batters shall be track rolled or stabilised by other means acceptable to the Principal Certifier. Appropriate Erosion and Sediment Controls shall be implemented. | <i>Erosion Control</i> |
| 4. Where seeding of stockpiles to encourage vegetation cover is specified, such work shall be carried out in accordance with Specification C273 LANDSCAPING. | <i>Seeding Stockpile</i> |

CUTTINGS

C213.11 SCOPE

- | | |
|---|-----------------------|
| 1. Construction of cuttings shall include all operations associated with the excavation of material within the limits of the batters including benching, treatment of cutting floors and transition from cut to fill. | <i>Extent of Work</i> |
|---|-----------------------|

C213.12 EXCAVATION

- | | |
|--|--------------------------------|
| 1. Materials encountered in cuttings shall be loosened and broken down as required so that they are acceptable for incorporation in the Works. In this regard, the Principal Contractor's attention is drawn to Clauses C213.23, C213.26, C213.28, C213.29, C213.30 and C213.31. | |
| 2. Cuttings shall have batter slopes as shown on the Approved Plans or as redetermined by the Principal Certifier on the basis of site inspection and investigation during the excavation. | <i>Batter Slopes</i> |
| 3. If necessary for safety or stability, the Principal Certifier, following advice from a qualified Geotechnical Engineer, may redetermine the batter slope of any section of a cutting after it has been completed in accordance with this clause. | <i>Change of Batter Slopes</i> |
| | <i>Hold Point</i> |

4. The tops of all cuttings shall be neatly "rounded".
5. In all cuttings, undulations in the general plane of the batter shall not be permitted except that batters will generally require progressive flattening at the ends of cuttings due to the presence of less stable material. False batters, cut batter extensions or noise mounds shall not be accepted without prior written approval by Council. *Batters to be Even*
6. Cut faces shall be cleaned of loose or unstable material progressively as the excavation proceeds. *Unstable Material*
7. Where, after the removal of Topsoil as specified in Clause C213.07, material of variable quality or moisture content is encountered, the Principal Contractor shall adjust the excavation methods to ensure blending of the materials, to obtain material meeting the requirements of Clause C213.23. *Blending Material*

C213.13 BATTER TOLERANCES

1. The tolerances for the excavation of batters, measured at right angles to the design grade line, shall be $\pm 300\text{mm}$. A survey conformance report shall be provided as part of the quality conformance records. The survey shall be taken at the top and toe of the batter at 20m intervals. *Batter Tolerances*
2. If the Principal Contractor excavates the batter beyond the batter slope line and the tolerance applicable thereto, the Principal Certifier may authorise a minor change in the general slope of the batter to suit the convenience of the Principal Contractor, but such a change shall not be regarded as a redetermination of the batter slope under Clause C213.11. The cost of any increase in excavation quantities resulting from such change in batter slope shall be borne by the Principal Contractor. Alternatively the Principal Contractor shall submit details of the material and/or methods proposed to restore the specified slope and stability of the batter for the Principal Certifiers approval. *Excavation beyond Batter Line*
3. For batters steeper than 1:1, if any section of the batter up to a height of 3m above the table drain level has been over excavated beyond the tolerance limit specified, the Principal Certifier may direct that the batter be restored to the average batter slope using randomly mortared stone. The stone shall be similar to the sound rock in the cutting and the mortar shall be coloured to match the colour of the rock. *Restoration of Batter Slope*

C213.14 BENCHING IN CUTTINGS

1. Cut batters shall be benched as shown on the Approved Plans to provide drainage and erosion control on cut batters. Notwithstanding the tolerances permitted under Clause C213.12, bench widths shall not be less than those shown on the Approved Plans. *Bench Construction*
2. Benches shall be maintained and cleaned of loose stones and boulders regularly throughout the Subdivision works. *Bench Maintenance*

C213.15 TREATMENT OF FLOORS OF CUTTINGS

1. The floors of cuttings shall be excavated, parallel to the designed grade line, to a designed floor level which shall be at the underside of the selected material zone or where there is no selected material zone, to the underside of the pavement subbase. The floors shall then be trimmed to a level of not more than 50 mm above or below the designed floor level. Where the Principal Certifier considers that any underlying material is unsuitable for pavement support, the Principal Certifier may direct that it be removed in accordance with Clause C213.21.

*Excavation
Level*

A qualified geotechnical engineer shall inspect the cutting floor to determine the appropriate foundation treatment. The inspection shall review features such as water ingress, cracks and fissures within rock and the presence of organic material.

Hold Point

2. The Principal Contractor shall rip or loosen all material in the floor to a minimum depth of 200mm below the designed floor level for the width of the selected material zone (or subbase layer, where no selected material zone). The maximum dimension of any particles in the ripped or loosened zone shall not exceed 150mm.

*Floor Material
Ripped*

3. Prior to ripping or loosening the cutting floor the Principal Contractor shall determine the CBR of the material in the floor by AS 1289.6.1.1. Sufficient tests shall be taken to represent all the various materials which may exist in the cutting floor. If material in the floors of cuttings has a CBR value less than the value quoted in Annexure C213A, the material shall be removed and replaced with material that is compliant or stabilised in-situ with the design assumptions of Specification D2 PAVEMENT DESIGN.

CBR Testing

4. Ripped or loosened material shall be made available for inspection by the Principal Certifier before recompaction commences. It shall be recompacted in accordance with Clause C213.36.

Recompaction

5. After recompaction, the floors of cuttings shall be re-trimmed parallel with the finished wearing surface so that their levels do not vary more than 0mm above or 40 mm below the designed floor levels.

*Level
Tolerances*

6. Prior to placing any subsequent layers over the completed cutting floor, the Principal Certifier shall ensure compliance with C213.36, C213.37 and C213.45.

*Inspection by
Principal
Certifier*

C213.16 TRANSITION FROM CUT TO FILL

1. After the removal of topsoil and before the excavation of any cutting commences the Principal Contractor shall survey and mark the position of the intersection line between cutting and embankment occurring at the underside of the selected material zone or pavement subbase.

*Intersection
Line*

2. Following excavation to the cutting floor, a terrace shall be excavated for the width +1m in either direction of the selected material zone (or subbase layer, where no selected material zone) to a depth of 600mm below the cutting floor and parallel to the finished design road surface, as shown in Figure C213.1.

*Terrace
Construction*

3. The terrace shall extend into the cut to the point where the cutting floor is 600mm below the original stripped surface, or a distance of 10 metres either side,

*Extent of
Terrace*

whichever is the lesser. The furthest extent of the cut to fill transition shall be excavated perpendicular to the alignment of the road.

4. The material excavated shall be either incorporated in the embankments or spoiled as directed by the Principal Certifier.

**Excavated
Material**

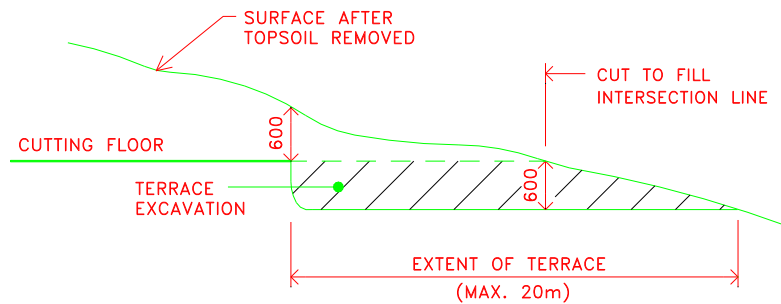


Figure C213.1 - Transition from Cut to Fill

5.

The material placed above the terrace shall satisfy the requirements of Clause C213.23 and shall be compacted in accordance with Clause C213.36.

Quality and Compaction

BLASTING

C213.17 GENERAL

1.

When explosives are permitted to be used, the Principal Contractor shall obtain all necessary licences from the appropriate authorities, and shall comply with all Government regulations relating to transport, storage, handling and the use of explosives and also to the rules set out in AS2187, Parts 1 and 2. The transport of explosives shall be in accordance with the Australian Code for the Transport of Explosives by Rail and Road. The requirements of the Environment Protection Authority (EPA) shall be complied with.

Principal Contractor to Obtain Licences
2.

The Principal Contractor shall be liable for any accident, damage or injury to any person, property or thing, resulting from the use of explosives.

Principal Contractor's Responsibility
3.

Before the start of blasting operations, the Principal Contractor, in the presence of the Principal Certifier, shall conduct a survey to determine and record the existing condition of all structures likely to be affected by any blast.

Pre-blast Survey
4.

Structures shall include public utilities. The survey shall include all structures within 500m of any blast but shall be extended where the maximum instantaneous charge proposed is likely to produce peak particle velocities greater than allowable at structures more remote from a blast site. A written report of the survey, supported by photographs where necessary, together with a list of any existing defects in the structures, shall be submitted to the owner of each structure and to the Principal Certifier before blasting commences.

Extent of Survey
5.

The Principal Contractor shall advise the Principal Certifier of the proposed maximum instantaneous charge and the Principal Contractor's validation of the adequacy of the proposed structural survey at least three working days before the survey is due to commence. The Principal Certifier may direct amendments to the scope of the survey as a result of blast monitoring during the work.

Amendment to Extent of Survey
6.

Before each blasting operation, the Principal Contractor shall submit to the Principal Certifier written details of the proposed blasting procedure including the quantity and type of explosive to be detonated, the blasting pattern to be used and measures proposed to limit noise and to ensure that vibration from blasting does not adversely affect nearby structures.

Proposed Blasting Procedure
7.

Ground vibration caused by blasting shall not exceed the values of peak particle velocity listed in Table C213.1:

Ground Vibration

Point of Potential Damage (within 1km of blasting site)	Peak Particle Velocity
Completed and cured bridge structures or sub-structures (eg completed abutment),	25 mm/sec

Bridgeworks and structural retaining walls under construction,	20 mm/sec
Residential premises, schools, hospitals and other buildings	5 mm/sec (with 10% not to exceed 10 mm/sec)
Buildings or monuments of historical significance	2 mm/sec

Table C213.1 - Limiting Peak Particle Velocity

- | | |
|--|--|
| <p>8. The Principal Contractor shall advise all residents within a radius of 1km, by letter drop before blasting operations commence, of the likely times, frequency and duration of blasting and precautions being taken to ensure that damage to property will not result.</p> | <p><i>Advice to Residents</i></p> |
| <p>9. Unless otherwise approved, blasting operations shall be confined to the periods Mondays to Fridays (excluding public holidays), 9am to 3pm. Council shall be notified in writing seven (7) working days before each blast.</p> | <p><i>Time Limits</i></p> <p><i>HOLD POINT</i></p> |
| <p>10. When blasting operations are being carried out, precautions shall be taken relating to the safety of persons and animals and the road shall be closed to traffic and the appropriate signs erected in accordance with Specification C201 CONTROL OF TRAFFIC.</p> | <p><i>Safety Precautions</i></p> |

C213.18 PRESPLITTING

- | | |
|--|-----------------------------------|
| <p>1. Where presplitting is carried out the spacing of presplit drill holes shall not exceed 750mm centre to centre.</p> | <p><i>Presplitting</i></p> |
|--|-----------------------------------|

C213.19 BLASTING RECORDS

- | | |
|--|---|
| <p>1. The Principal Contractor shall maintain accurate records of each blast showing the details listed below:-</p> <ul style="list-style-type: none"> (a) Date and time of blast (b) Location, number and diameter of holes loaded (c) Depth of each hole loaded (d) Inclination of holes (e) Maximum and minimum burden (f) Types of explosives used (g) Charge distribution in each hole (h) Maximum instantaneous charge (i) Delay periods and sequence (j) Total amount of charges in the blast (k) Length and type of stemming in each hole | <p><i>Records to be kept</i></p> |
|--|---|

2. The records shall be prepared as holes are loaded and signed by the Powderman. A copy shall be provided to the Principal Certifier on the day of the blast.

*Record
Preparation*

C213.20 CONTROL OF AIR BLAST OVER-PRESSURE

1. The Clause shall apply only where a noise sensitive location exists within 1km of the blasting site.

Incidence

2. The Principal Contractor(s) shall comply with noise regulated under the Protection of the Environment Operations Act (POEO) Act.

3. The noise emanating from blasting operations shall not exceed an over-pressure level of 115 decibels (linear peak) at any noise sensitive location (such as residential premises, schools or hospitals). Up to 10 per cent of the total number of blasts may exceed this value provided a level of 120 decibels is not exceeded at any time.

*Noise
Limitations*

4. The Principal Contractor shall arrange for the monitoring of air blast over-pressure to ensure compliance with the specified limits. All monitoring shall be carried out by personnel possessing current NATA registration for such monitoring. All test results shall be reported on NATA endorsed test certificates which shall include a clear statement as to compliance or non-compliance with the requirements of this Specification. In general, a monitoring location will be near the perimeter of the noise sensitive location at the point closest to the maximum charge. The Principal Contractor shall submit a copy of the monitoring record to the Principal Certifier.

*Monitoring of
Air Blast Over-
Pressure*

5. In the event that the measured air blast over-pressure exceeds the specified limits, the Principal Contractor shall suspend further blasting work and shall submit to the Principal Certifier proposals detailing any additional steps and precautions the Principal Contractor shall take to ensure that for any future blast, the limiting over-pressure shall not be exceeded. The Principal Contractor shall not resume any blasting until such proposals have been submitted.

*Excessive Air
Blast Over-
Pressure*

C213.21 CONTROL OF GROUND VIBRATION

1. The Principal Contractor shall arrange for the monitoring of ground vibrations to ensure compliance with the peak particle velocity limits shown in Table C213.1. All monitoring shall be carried out by personnel possessing current NATA registration for such monitoring. All test results shall be reported on NATA endorsed test certificates which shall include a clear statement as to compliance or non-compliance with the requirements of this Part of the Specification. In general a monitoring location shall be near the perimeter of the structure or building at the point closest to the maximum charge. The Principal Contractor shall submit a copy of the monitoring record to the Principal Certifier.

*Monitoring
Vibrations*

2. To minimise the risk of peak particle velocity limits being exceeded, the Principal Contractor shall develop a blasting site relationship between peak particle velocity, distance and blasting charge as set out below.

*Blasting Site
Relationship*

3. For the first blast, monitors shall be set up at not less than five points at varying distances away from the blasting site. The Maximum Instantaneous Charge for the first blast shall not exceed that calculated from the following formula:

**Maximum
Instantaneous
Charge**

$$MIC = 0.5 \left[\frac{D}{\left[\frac{p.p.v.}{1140} \right]^{-0.625}} \right]^2$$

where MIC = Maximum Instantaneous Charge in kilograms

D = Distance in metres from charge to the point of potential damage

PPV = limiting peak particle velocity from Table C213.1

4. A log-log (base 10) graph of measured peak particle velocity (vertical axis) versus Scaled Distance (horizontal axis) shall be plotted, where

$$\text{Scaled Distance} = \frac{D}{\sqrt{MIC}}$$

The mean regression line shall be obtained by the least squares method.

5. For subsequent blasts, the MIC and other aspects of blast design may be adjusted provided that further ground vibration monitoring is undertaken and the mean regression line redetermined to demonstrate that peak particle velocity limits are not exceeded. The Principal Contractor shall make the regression line plots available to the Principal Certifier, if so requested.

**Adjustment of
Blast Design**

UNSUITABLE MATERIAL

C213.22 GENERAL

1. Unsuitable material is that occurring below the designed floor level of cuttings and below the nominated depth for stripping topsoil beneath embankments, which the Principal Certifier in conjunction with a qualified Geotechnical Engineer deems to be unsuitable for embankment or pavement support in its present position. Unsuitable material also includes material in cuttings which the Principal Certifier in conjunction with a qualified Geotechnical Engineer deems to be unsuitable for embankment construction.

Definition

HOLD POINT

2. Such material shall be excavated to the extent directed by the Principal Certifier and Geotechnical Engineer. Material removed as unsuitable shall, as directed by the Principal Certifier and Geotechnical Engineer, be incorporated in embankments in accordance with Clause C213.23 or spoiled in accordance with Clause C213.34.

**Extent of
Excavation**

3. After removal of the unsuitable material, the floor of the excavation shall be represented to the Principal Certifier for inspection, prior to backfilling with replacement material, to determine whether a sufficient depth of unsuitable material has been

**Floor
Inspection**

removed. Prior to placing replacement material the excavated surface shall be compacted in accordance with Clause C213.36.

**WITNESS
POINT**

4. The unsuitable material which has been removed shall be replaced with material from cuttings, or with material borrowed in accordance with Clause C213.35, of the quality specified in Clause C213.23. Replacement material is deemed to form part of embankment construction. It shall be placed in accordance with Clause C213.26 and compacted in accordance with Clause C213.36.

**Replacement
Material**

EMBANKMENT CONSTRUCTION

C213.23 SCOPE

1. Embankment construction includes all operations associated with the preparation of the foundation areas on which fill material is to be placed, the placing and compacting of approved material within areas from which unsuitable material has been removed in accordance with Clause C213.21, the placing and compacting of fill material and of materials of specified quality in nominated zones throughout the Works and all other activities required to produce embankments as specified to the alignment, grading and dimensions shown on the Approved Plans. It also includes any pretreatment such as breaking down or blending material or drying out material containing excess moisture.

Extent of Work

C213.24 EMBANKMENT MATERIAL

1. Material for embankment construction shall be obtained from the cuttings within the Works, in accordance with Clause C213.11 supplemented by borrow in accordance with Clause C213.35 and from other sources as approved by the Principal Certifier if necessary. The material shall be free of tree stumps and roots, clay, topsoil, steel, organic material and other contaminants and shall be capable of being compacted in accordance with Clause C213.36.

**Location and
Quality**

2. The work shall be programmed so that material of the quality specified in Clause C213.26 and C213.30 for the upper zones of the formation is available when required.

**Selection of
Material**

C213.25 FOUNDATIONS FOR EMBANKMENTS

1. Following removal of topsoil in accordance with Clause C213.07, the embankment foundation area shall be made available for inspection by the Principal Certifier. A qualified geotechnical engineer shall inspect the stripped surface to determine the appropriate foundation treatment. The inspection shall review features such as water ingress, cracks and fissures within rock and the presence of organic material.

Inspection

Hold Point

2. Where the Principal Certifier considers that any underlying material is unsuitable, the Principal Certifier may direct that it be removed and replaced in accordance with Clause C213.21.

**Unsuitable
Material**

a) Foundations for Shallow Embankments***Shallow
Embankments***

1. Shallow embankments are those embankments of a depth less than 1.0 metre from the top of pavement to stripped surface. After removal of topsoil the Principal Contractor shall survey and work out the extent of the area of shallow embankments.

2. Material in the foundations for shallow embankments which does not meet the requirements specified in Annexure C213A, shall be deemed unsuitable in accordance with Clause C213.21 and shall be replaced by material of the specified quality.

***Unsuitable
Material***

3. Foundations for shallow embankments shall be prepared for embankment construction after removing topsoil and unsuitable, by loosening the material exposed to a depth of 200mm, adjusting the moisture content of the loosened material and compacting as specified in Clause C213.36. The Principal Contractor shall use equipment and techniques to minimise surface heaving or other foundation damage.

***Preparation of
Foundations*****b) Other Embankments**

1. For all other embankments the foundation shall be prepared by grading and levelling the general area, adjusting the moisture content where necessary and compacting the top 200mm as specified in Clause C213.36.

Preparation

2. Where a bridging layer has been specified as a foundation treatment in the approved design plans, it shall be supplied and placed as part of General Earthworks. The bridging layer shall consist of free-draining granular material which shall be end-dumped and spread in a single layer in sufficient depth to allow the passage of earthmoving equipment with minimal surface heaving. The compaction requirements of Clause C213.36 shall not apply to the bridging layer. It may be necessary to import suitable material from off site if no suitable borrow source is available as provided in Clause C213.35.

Bridging Layer

3. A bridging layer may also be employed, subject to the approval of the Principal Certifier, where ground water or seepage is encountered in the foundation area or where the Principal Contractor demonstrates that it is impracticable to achieve the degree of compaction specified for the foundation in Clause C213.36. A bridging layer shall not be acceptable if its proximity to the pavement is likely to affect the pavement design.

***Seepage from
Foundations*****C213.26 HILLSIDE EMBANKMENTS**

1. Where embankments are to be constructed on or against any natural slopes or the batters of existing embankments, the existing slope or batter, if it is steeper than 4 horizontal to 1 vertical in any direction shall be cut in the form of horizontal terraces over the whole area to be covered by new filling. The existing slope or batter shall be stepped in successive terraces, each at least 1 metre in width and maximum 1m in height, the terraces to be cut progressively as the embankment is placed. Wherever possible terraces shall coincide with natural discontinuities. Subsoil drainage may be required in some instances. Material thus excavated shall be compacted as part of the new embankment material.

***Horizontal
Terraces***

C213.27 PLACING FILL FOR EMBANKMENT CONSTRUCTION

- | | |
|---|---|
| <p>2. The methods of excavation, transport, depositing and spreading of the fill material shall be selected so as to ensure that the placed material is uniformly mixed.</p> | <p><i>Uniformity of Material</i></p> |
| <p>3. The embankment shall be constructed so as to derive its stability from the adequate compaction of the fine material embedding the large rock pieces rather than mechanical interlock of the rock pieces. The fine material shall be compacted to meet the requirements of Clause C213.36.</p> | <p><i>Embankment Stability</i></p> |
| <p>4. Fill material for embankment construction shall be placed in layers parallel to the grade line and compacted in accordance with Clause C213.36. The layers shall be of uniform compacted thickness not exceeding 200mm, except that where more than 25 per cent by volume of the filling consists of rock with any dimension larger than 150mm, the Principal Certifier may approve an increase in the compacted layer thickness to 300mm, provided that the relative compaction specified in Clause C213.36 is attained.</p> | <p><i>Layer Thickness</i></p> |
| <p>5. The maximum dimension, measured in any direction, of rock pieces in the fill material for embankment construction shall not exceed two-thirds of the approved compacted layer thickness. Any larger rock pieces shall be reduced in size for incorporation in the embankment layers.</p> | <p><i>Maximum Size Rock Pieces</i></p> |
| <p>6. Rock material shall be broken down and evenly distributed through the fill material, and sufficient fine material shall be placed around the larger material as it is deposited to fill the voids and produce a dense, compact embankment. Where the Principal Certifier considers insufficient fine material is present to fill the voids, additional fine material shall be obtained from other places in the work or by a change in the method of winning fill material.</p> | <p><i>Grading of Fill Material</i></p> |
| <p>7. Stony patches with insufficient fine material to fill the voids shall be reworked with additional fine material being blended in to achieve a dense, compact layer.</p> | <p><i>Reworking Stony Patches</i></p> |
| <p>8. In placing embankment layers, the Principal Contractor shall use equipment and techniques to avoid surface heaving or other damage to the foundations and underlying embankment layers.</p> | <p><i>Equipment Selection for Placement</i></p> |
| <p>9. After compaction, embankment material in the zone(s) below the selected material zone (or subbase layer, where no selected material zone) shall have a CBR value not less than that quoted in Annexure C213A for the depth(s) specified in Annexure C213A.</p> | <p><i>CBR Value</i></p> |
| <p>10. For the purpose of this Clause, the CBR value of the material shall be determined by Test Method AS 1289.6.1.1.</p> | <p><i>Test Methods</i></p> |
| <p>11. The Principal Contractor shall be responsible for determining suitable sources of material and for any processing to satisfy these quality requirements.</p> | <p><i>Principal Contractor's Responsibility</i></p> |

C213.28 EMBANKMENT BATTERS

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|----|---|------------------------------|
| 1. | The batter slopes shown on the Approved Plans represent the estimated requirements for the expected types of materials, and may be subject to redetermination by the Principal Certifier according to the Principal Certifier's assessment of the materials encountered. | <i>Batter Slopes</i> |
| 2. | When completed, the average planes of the batters of embankments shall conform to those shown on the Approved Plans or as determined by the Principal Certifier. No point on the completed batter shall vary from the specified slope line by more than $\pm 300\text{mm}$, when measured at right angles to the grade line. However, in no case shall the edge of the formation at the underside of the pavement be nearer to the roadway than shown on the Approved Plans. | <i>Slope
Tolerances</i> |
| 3. | Undulations in the general plane of the batter shall not be permitted. | <i>Slope
Undulations</i> |

C213.29 ROCK FACING OF EMBANKMENTS

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| 1. | Where shown on the Approved Plans, embankment batters (including embankments at bridge abutments) shall be provided with a facing of clean, hard, durable rock. | <i>Extent</i> |
| 2. | The rock facing shall be built up in layers ahead of each layer of filling. Rock may be placed by hand or plant but shall be placed in such a manner that its least dimension is vertical and that mechanical interlock between the larger stones occurs. Any rock deposited in the rock facing which has an excess of fine material surrounding it shall be removed together with the excess fine material and replaced. | <i>Mechanical
Interlock</i> |
| 3. | The Principal Contractor shall adjust his working methods and programme the work so as to obtain hard and durable rock of the specified dimensions as it is required. The space between larger batter rocks shall be filled with progressively smaller rocks to form a 'graded filter' which prevents the leaching out of fines from the fill material but which does not overfill the voids between larger rocks, or cause the larger rocks to lose contact with one another. Fine material shall not cover the outside of the rocks on the face of the batter. | <i>Graded Filter</i> |
| 4. | The Principal Contractor shall exercise extreme caution whilst placing the rock facing. Where embankment material is placed above other roads in use the outer rock layer shall be placed in such a manner as to prevent spillage down the batter. The Principal Contractor shall ensure that, under no circumstances, could any rock be dislodged and roll onto any adjacent roadway or track in use. | <i>Caution in
Placement</i> |

C213.30 TRIMMING TOPS OF EMBANKMENTS

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| 1. | The tops of embankments shall be trimmed parallel to the designed grade line at levels equal to the finished surface level less the thicknesses of pavement courses and the selected material zone. | <i>Levels</i> |
| 2. | The tops of embankments at these levels shall be compacted to meet the requirements of Clause C213.36 and trimmed so that they do not vary more than 0mm above or 40 mm below the levels as calculated above. | <i>Tolerances</i> |

C213.31 SELECTED MATERIAL ZONE

1. A selected material zone shall be provided as indicated on the Approved Plans and in accordance with the following quality requirements:

Dimension and Quality

- (a) it shall be free from stone larger than 100mm maximum dimension
- (b) the fraction passing 19.0mm AS sieve shall have a CBR value of not less than that quoted in Annexure C213A.

2. The selected material shall be obtained from cuttings excavated under the Contract or from borrow areas as specified in Clause C213.35. If necessary, the Principal Contractor shall use working methods to yield material for the selected material zone by breaking down oversize rock or by other means, including processing through a crusher, to ensure that the resulting material conforms to the requirements of this Clause.

Winning Material

3. The Principal Contractor shall ensure that any material encountered of the quality specified for the selected material zone shall be either placed directly in the selected material zone or stockpiled at locations approved by the Principal Certifier for future use by the Principal Contractor in the selected material zone until at least sufficient material is reserved to complete the selected material zone over the whole work.

Selection of Material

4. The selected material zone shall be placed and compacted in layers with the compacted thickness of each layer not exceeding 150mm. Compaction shall be as specified in Clause C213.36.

Layer Thickness

5. After placement the selected material shall be homogeneous and free from patches containing segregated stone or excess fines. There shall be no areas containing material which does not comply with the specified requirements of this Clause.

Homogeneous Layers

6. The top of the selected material zone shall be compacted and trimmed parallel with the designed grade line at a level equal to the finished surface level minus the thickness of pavement layers adopted. The tolerances for the trimmed levels are given in Annexure C213A.

Tolerances

Prior to placing any subsequent pavement layers over the completed select material zone surface, the Principal Certifier shall ensure compliance with C213.36, C213.37 and C213.45. Furthermore a proof roll shall be performed by the Principal Certifier. The proof roll shall be undertaken with a dual wheel truck with minimum axle weight of 8 tonnes which does not reveal any visible deflection.

C213.32 FILL ADJACENT TO STRUCTURES

1. For the purpose of this Clause, structures shall include bridges, precast and cast-in-place box culverts and retaining walls. Fill adjacent to other culverts and drainage structures shall be provided in accordance with the particular Specifications for STORMWATER DRAINAGE as appropriate.

Structure Types

2. No filling shall be placed against structures within 21 days after placing concrete in these structures, unless the walls are effectively supported by struts to the satisfaction of the Principal Certifier, or when the Principal Contractor has demonstrated to the Principal Certifier that 75 per cent of the design strength of the concrete has been achieved.

C213.33 TREATMENT AT WEEPHOLES

1. Drainage adjacent to weepholes shall be provided by either a layer of broken stone or river gravel consisting of clean, hard, durable particles graded from 50mm to 10mm such that:

Grading

- (a) The maximum particle dimension shall not exceed 50mm
- (b) No more than 5 per cent by mass shall pass the 9.5mm A.S. sieve.

2. The broken stone or river gravel shall be continuous in the line of the weepholes, extend at least 300mm horizontally into the fill and extend at least 450mm vertically above the level of the weepholes.

Extent

3. Alternatively the Principal Contractor may provide a synthetic membrane of equivalent drainage characteristics. It shall be stored and installed in accordance with Manufacturer's instructions. The use of a synthetic membrane shall be subject to the Principal Certifier's approval.

Synthetic Membrane

C213.34 SELECTED BACKFILL

1. Selected backfill shall be placed adjacent to structures in accordance with Table C213.2. The selected backfill shall consist of a granular material having a maximum dimension not exceeding 50mm and a Plasticity Index, determined by AS 1289.3.3.1, neither less than 2 nor more than 12.

Quality

Structure Type	Selected Backfill	
	Width	Height
Bridge abutments	2m	H
Cast-in-place Box Culverts	H/3	H + 300mm
Corrugated Steel Pipes and Arches	0.5m	H + 500mm
Retaining walls	H/3	H

(Where H = height of structure)

Table C213.2 - Selected Backfill, Width and Height

2. The selected backfill shall be placed in layers, with a maximum compacted thickness of 150mm. Layers shall be placed simultaneously on both sides of box culverts to avoid differential loading. Compaction shall start at the wall and proceed away from it, and shall meet the requirements of Clause C213.36.

Placement in Layers

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| 3. The existing embankment slope behind the structure shall be cut in the form of successive horizontal terraces, each terrace being at least 1 metre in width, and the selected backfill shall be placed in accordance with Clause C213.26. | Horizontal Terraces |
| 4. No selected backfilling shall be placed against structures, retaining walls, headwalls, or wingwalls within 21 days after placing of the concrete, unless the walls are effectively supported by struts to the satisfaction of the Principal Certifier, or when the Principal Contractor has demonstrated to the Principal Certifier that 85 per cent of the design strength of the concrete has been achieved. | Time of Placement |
| 5. Where a bridge deck is being concreted adjacent to an abutment, no filling shall be placed against the abutment within twenty-one days after placing concrete in the bridge deck, unless approved by the Principal Certifier. | Adjacent to Concrete Deck |
| 6. In the case of spill-through abutments, rocks shall not be dumped against the columns or retaining walls but shall be built up evenly by hand placing around or against such structures. | Spill through Abutments |
| 7. In the case of framed structures, embankments at both ends of the structure shall be brought up simultaneously, the difference between the levels of the embankments at the respective abutments, shall not exceed 500mm. | Framed Structures |

C213.35 SPOIL

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|--|---------------------------|
| 1. Spoil is surplus material from excavations under the Contract which is not required to complete the Works as specified or material from excavations under the Contract whose quality the Principal Certifier deems to be unacceptable for incorporation in the Works. | Definition |
| 2. Where there is surplus material the Principal Certifier in consultation with the Principal Consultant may seek approval from Council to flatten batter slopes for embankments which have not been commenced, and/or direct that the excess material be used in the uniform widening of embankments, the surface of which shall be shaped so as to provide a tidy appearance and effective drainage. The surplus material shall be spread and compacted as specified in Clauses C213.26 and C213.36 for material in embankments. | Use in Embankments |
| 3. Alternatively, spoil shall be disposed of in accordance with the relevant waste regulations. The cost of transport and disposal of all spoil material shall be borne by the Principal Contractor. | Disposal of Spoil |

C213.36 BORROW

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|---|------------------------------------|
| 1. Where borrow material is required to complete the Works as specified, the location of borrow sites shall be as approved by the Principal Certifier, with prior approval by Council, and the quality of material shall be acceptable to the Principal Certifier in accordance with Clauses C213.23, C213.28 or C213.31 as appropriate. The edges of borrow sites shall be no closer than 3 metres from any fence line, or edge of excavation or embankment. Adequate clearance shall be provided for the construction of catch drains. Borrow sites shall have drainage outlets acceptable to the Principal Certifier, cut batter slopes not steeper than 4 to 1, and shall be left by the Principal Contractor in a tidy and safe condition. | Borrow Site Characteristics |
|---|------------------------------------|

- | | |
|--|--|
| <p>2. For borrow within the defined working area for the Works as specified, site preparation shall be in accordance with Specification C212 CLEARING AND GRUBBING and Clause C213.07. Restoration of borrow sites shall be carried out in accordance with Specification C273 LANDSCAPING.</p> | <p><i>Site
Preparation
and
Restoration</i></p> |
| <p>3. If borrow material is obtained by uniformly widening a cutting, the requirements of Clauses C213.11, C213.12 and C213.14 as to the redetermination of batter slopes, the trimming of batters and the compaction of floors of cuttings respectively shall apply to the borrow area.</p> | <p><i>Widening of
Cutting</i></p> |
| <p>4. If the Principal Certifier accepts that borrow has to be obtained from locations outside the specified working area for the Works, the Principal Contractor shall be responsible for obtaining any permits required for entry on land and for the payment of any royalty for such borrow material. The Principal Contractor shall also comply with any requirements of the Environmental Planning and Assessment Act, the Local Council, land owners, the Rural Lands Protection Board and the NSW Department of Land and Water Conservation and other relevant legislation.</p> | <p><i>Principal
Contractor
Responsibility</i></p> |

COMPACTION AND QUALITY CONTROL

C213.37 COMPACTION AND MOISTURE REQUIREMENTS

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| <p>1. In areas listed below, all layers shall be uniformly compacted to not less than the relative compaction specified before the next layer is commenced. Each layer of material shall be trimmed prior to and during compaction to avoid bridging over low areas. A smooth surface shall be presented at the top of each layer.</p> | <p><i>Trimming and
Compaction</i></p> |
| <p>2. The following areas shall be compacted to provide a relative compaction, determined by AS 1289.5.1.1 for standard compactive effort, of not less than 95%. Tests are to be performed for the full depth of the layer.</p> <ul style="list-style-type: none">(a) Each layer of material replacing unsuitable material as detailed in Clause C213.21.(b) Each layer of material placed in embankments, up to 0.5 metres from the top of the pavement.(c) The whole area on the floors of cuttings.(d) Fill placed adjacent to structures up to 1.0 metre from the top of pavement.(e) Material in unsealed verges and within medians up to the level at which topsoil is placed.(f) Spoil (excluding unsuitable material)(g) All other areas except those where 98 per cent relative standard compaction is specified. | <p><i>95% Standard
Compaction
Requirements</i></p> |
| <p>3. Unsuitable material shall be stockpiled as directed by the Principal Certifier and compacted by track rolling.</p> | <p><i>Unsuitable
Material</i></p> |

4. The following areas shall be compacted to provide a relative compaction, determined by AS 1289.5.1.1 for standard compactive effort, of not less than 98 per cent. Tests are to be performed for the full depth of the layer.

**98% Standard
Compaction
Requirements**

- (a) Foundations for shallow embankments.
- (b) Foundations other than shallow embankments.
- (c) Each layer of the embankment within 0.5 metres from the top of pavement.
- (d) Each layer of the selected material zone as specified in Clause C213.30.
- (e) Any areas of material of specified quality which may be shown on the Approved Plans or specified elsewhere behind kerbs and/or gutters or adjacent to rigid pavements.
- (f) The fill material placed adjacent to structures as specified in Clauses C213.31 and C213.33 in each layer within 1.0 metre from the top of the pavement.

5. At the time of compaction the moisture content of the material shall be adjusted so as to permit the specified compaction to be attained at a moisture content which, unless otherwise approved by the Principal Certifier, is within the range set out in Annexure C213A of the optimum moisture content as determined by AS 1289.5.1.1 or AS 1289.5.7.1. Material which becomes wetted up after placement shall not be compacted until it has dried out so that the moisture content is within this range. The drying process may be assisted by aeration, or where approved by the Principal Certifier, by the use of hydrated or quick lime. Alternatively the Principal Contractor may transport the wet material to a stockpile site for drying out and later use as fill material. The cost of transport to stockpile for drying out and later use shall be borne by the Principal Contractor.

**Moisture
Content**

If there is insufficient moisture in the material for it to be compacted as specified, water shall be added. The added water shall be applied uniformly and thoroughly mixed with the material until a homogeneous mixture is obtained. The cost of such wetting or drying the material to be compacted shall be borne by the Principal Contractor.

6. Compaction shall be undertaken to obtain the specified relative compaction for the full depth of each layer in embankments and for the full width of the formation over the entire length of the work. Compaction shall be completed promptly to minimise the possibility of rain damage.

**Prompt
Compaction**

7. Any material placed by the Principal Contractor that has attained the specified relative compaction but subsequently becomes wetted up so that the moisture content is greater than the apparent optimum, determined by AS 1289.5.7.1, shall be dried out and uniformly recompact to the required relative compaction in accordance with this Clause before the next layer of material is placed. Alternatively, the Principal Contractor may remove the layer of wetted material to a stockpile site for drying and later re-use. The cost of the removal to stockpile, drying out and reincorporation of the wet material shall be borne by the Principal Contractor.

**Moisture
Content above
Optimum**

C213.38 TEST LOCATIONS

1. The inspection and testing for the works shall be in accordance with Level 2 Inspection and Testing detailed in AS 3798-2007, unless the Principal Certifier directs Level 1 Supervision. *AS 3798 supervision level*
2. Frequency of field density tests shall be in accordance with Table 8.1 of AS 3798-2007. Testing shall incorporate all service and utility trench backfill, which includes electricity, telecommunications and gas trenches.
3. Sampling locations and frequency for testing shall be determined as described in the Specification Part for Quality Requirements. Prior to testing the Principal Contractor shall work the lot to ensure uniform moisture content and compaction of all material within the lot. *Principal Contractor to Prepare Area*
4. The tests then taken shall be considered to represent the total volume of material placed within the lot. *Test Representation*
5. Where the Principal Certifier considers that the material which is present has not achieved uniformity required by this Clause or Clause C213.26, the Principal Certifier may take or direct further testing. The Principal Certifier shall nominate the area represented by the additional testing. *Further Testing*
6. If such testing confirms that material not conforming to the Specification is present the cost of such tests shall be borne by the Principal Contractor. The Principal Contractor shall carry out remedial work as necessary to achieve conformance to the requirements of Clause C213.36. *Remedial work*

C213.39 DEFLECTION MONITORING

1. Following completion of the formation to the underside of the selected material zone in accordance with Clause C213.24 and C213.26, and completion of the selected material zone in accordance with Clause C213.30, the Principal Contractor shall make the work available in lots, for the Principal Certifier or Council to carry out deflection monitoring. *Timing of Deflection Monitoring*

Deflection monitoring shall consist of proof rolling with truck dual wheels with minimum axel weight of 8 tonnes which does not reveal any visible deflection.

2. A lot for deflection testing shall consist of a continuous length of formation of at least 300 m and a single carriageway width which is generally homogeneous with respect to material and appearance. The Principal Contractor shall identify the boundaries of each lot with stakes clearly labelled to the satisfaction of the Principal Certifier. *Lot Size*

C213.40 WIDENING OF FORMATION

1. Road shoulders and formation shall be widened to accommodate footpaths, guardfence, streetlight plinths, emergency telephone bays and vehicle standing areas as shown on the Approved Plans. *Provision for Services*

LIMITS AND TOLERANCES

C213.41 SUMMARY OF TOLERANCES

1. The tolerances applicable to the various clauses in this Specification are summarized in Table C213.3 below:

Item	Activity	Limits/Tolerances	Spec Clause
1.	Batter Slopes		
	a) Excavation	± 300mm	C213.12
	b) Embankment	± 300mm	C213.27
2.	Floors		
	a) Floor of Cutting	Parallel to the designed grade line and ± 50mm of the designed floor level prior to compaction Trimmed layer to be parallel to the designed grade line, 0mm or - 40mm of the levels specified.	C213.14
3.	Tops of Embankments		
	Trimming tops of Embankments	Parallel to the designed grade line, +0mm or -40mm of the levels specified	C213.29
4.	Selected Material	Annexure C213A	C213.30

NOTE: Plus (+) is towards the roadway/surface and minus (-) is away from the roadway/surface. Tolerances are measured at right angles to design surfaces.

Table C213.3 - Limits and Tolerances

ANNEXURE C213A

EARTHWORKS - SUPPLEMENTARY INFORMATION

CLAUSE	DESCRIPTION	VALUE
C213.07	The depth below natural surface up to which the removal and measurement of top soil shall apply:	
	a) Cutting areas	100 mm
	b) Embankment areas	100 mm

C213.14	Minimum CBR value in cutting floors used for design of pavement	3 %
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C213.24 Requirements of material in foundations for shallow embankments:

Moisture Content - within the range of 60% to 95% of optimum.

C213.26 Upper Zones of Formation
and

C213.30 Selected Material Zone

Material within each zone shall have a CBR value of not less than the following,
under the nominated test conditions:

Location	Minimum CBR Value	Nominated Soaking Period (Days)
a. Selected Material Zone	15	4
b. Material below Selected Material Zone to 1.0 metre from top of pavement	3	4

C213.30 Construction tolerances for Selected Material Zone are +0 mm or -40 mm of the designed grade and crossfall.

C213.36 Moisture Content of material placed in embankments:

(a)Material in upper zones of formation:- within the range of 60% to 95% of optimum.

(b)All other embankment material:- within the range of 60% to 95% of optimum.